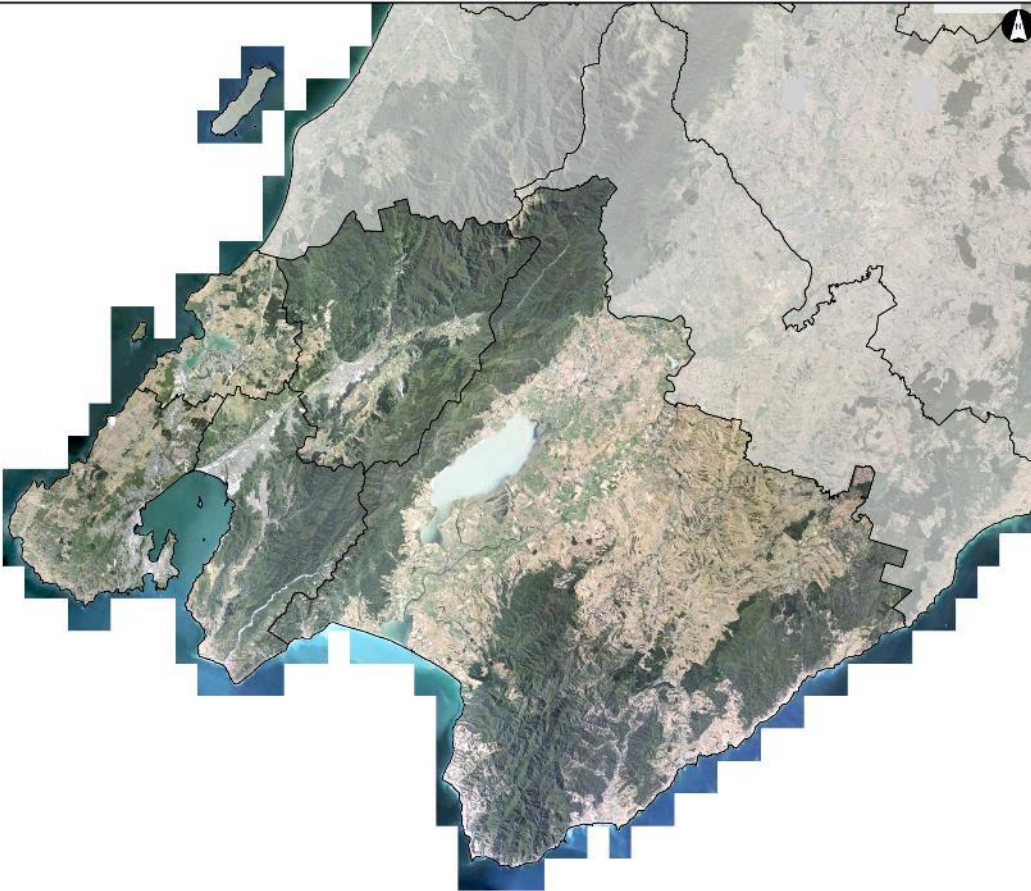


Wellington Region Runoff Hydrograph Calculator



Charlotte Lockyer

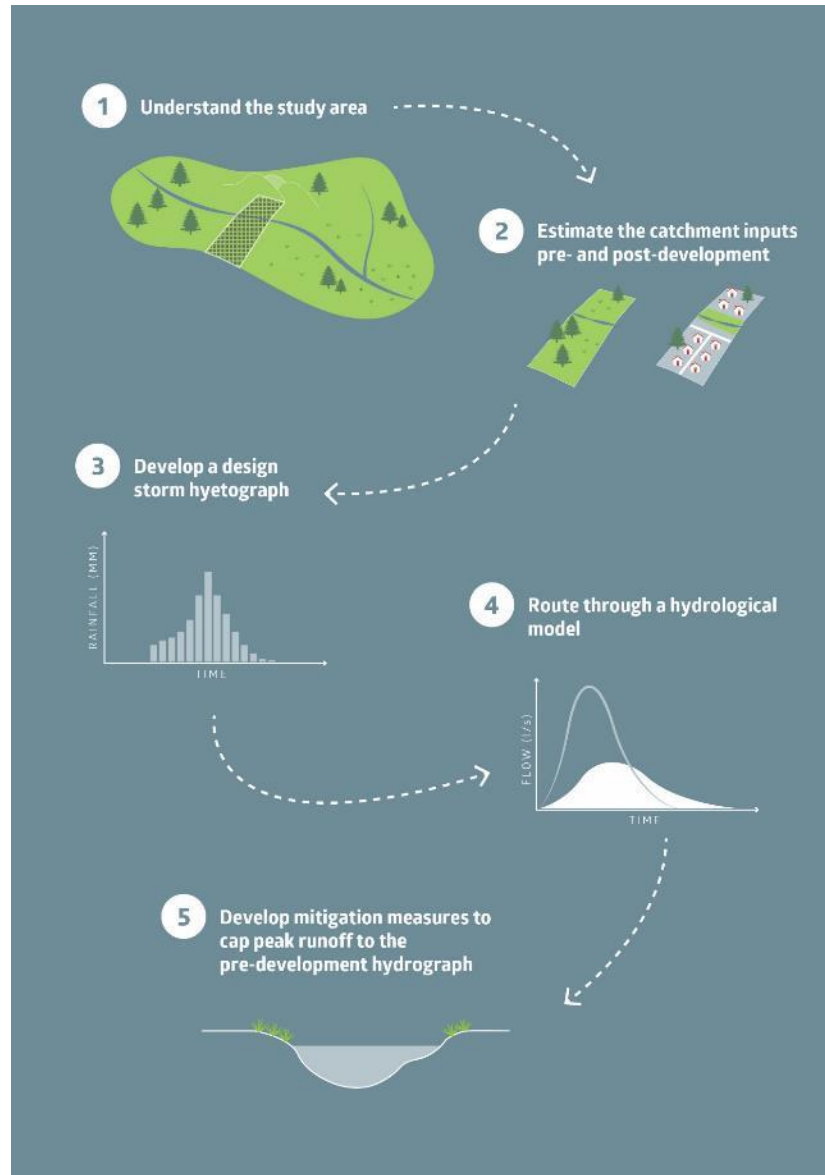


Agenda

1. Reference Guide Update
2. Runoff Hydrograph Calculator

Reference Guide update

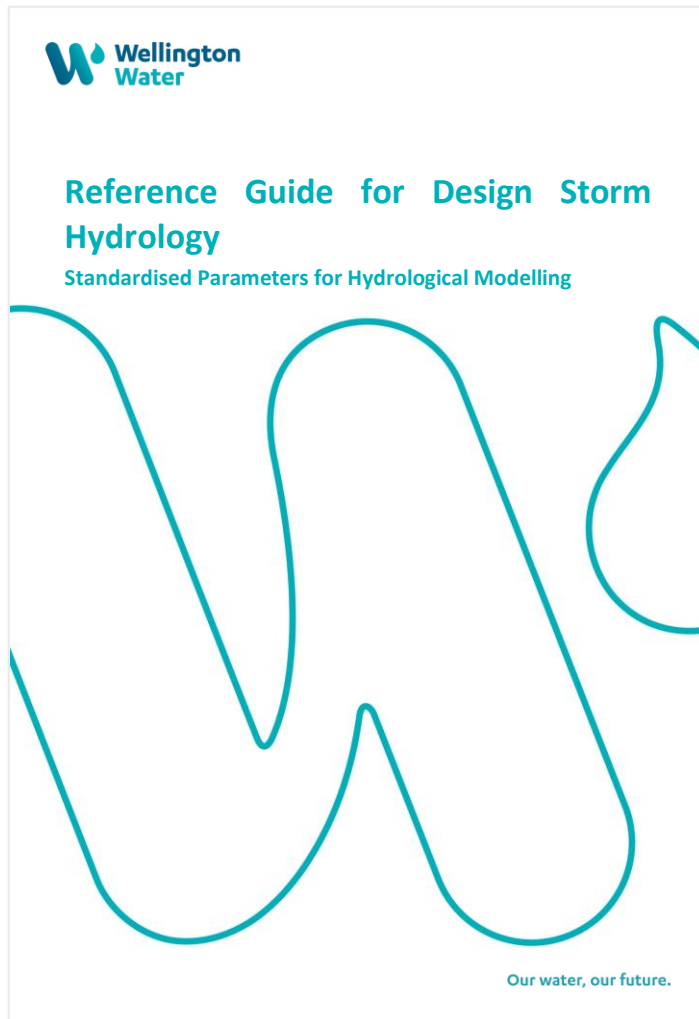
SCS Runoff Curve Number Method



Input parameters:

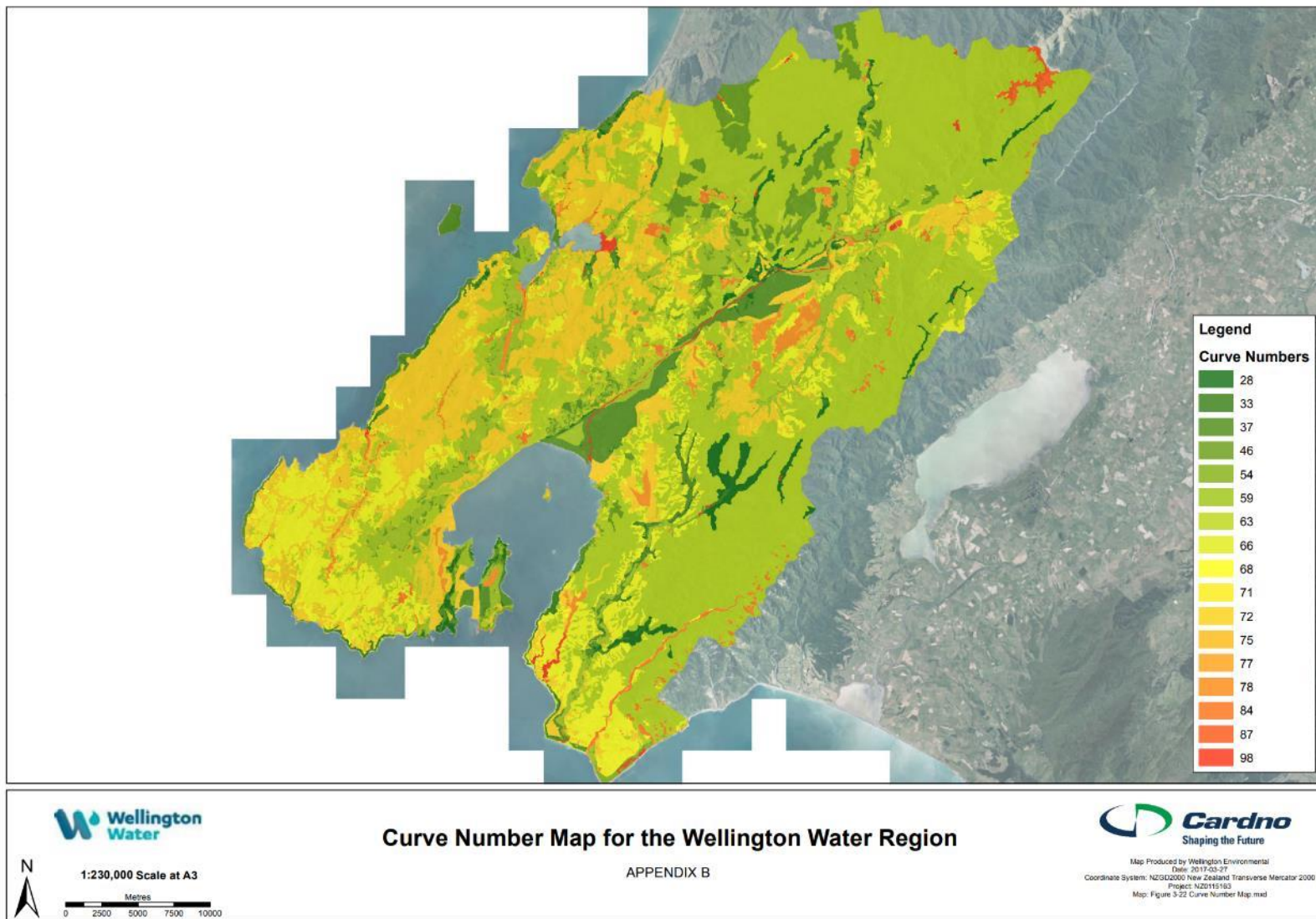
- Area
- Initial abstraction
- Curve number
- Time of concentration
- Rainfall

Three key changes

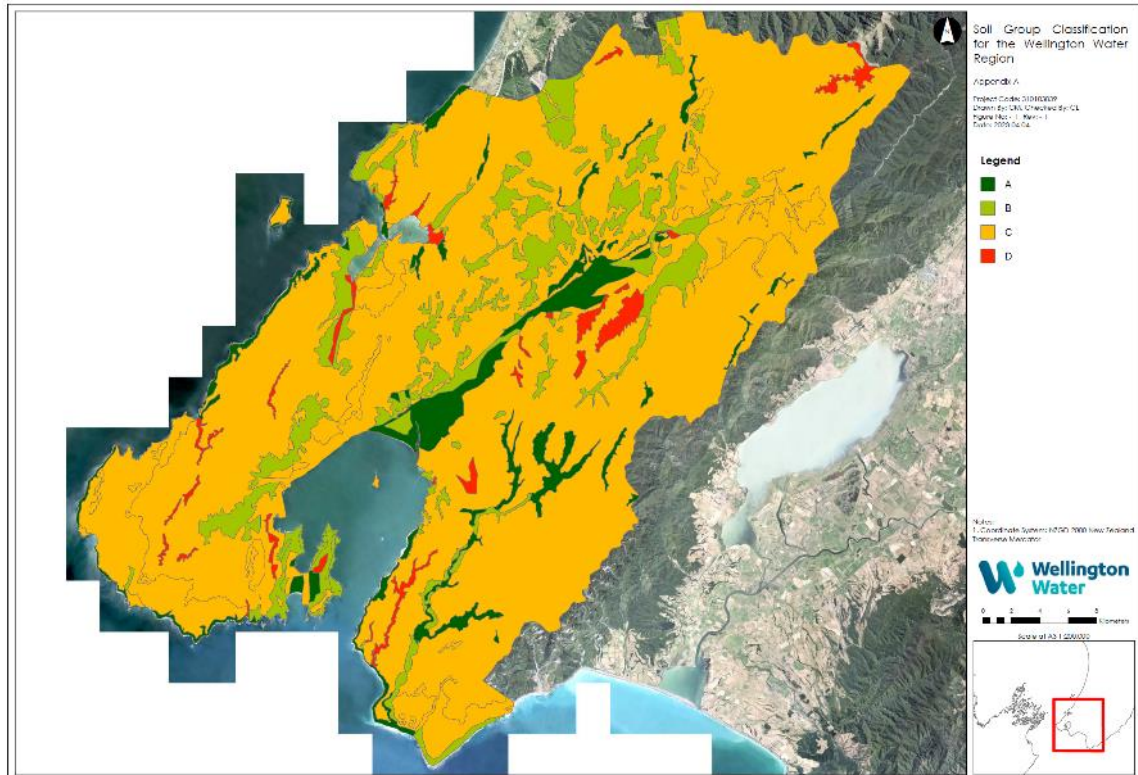
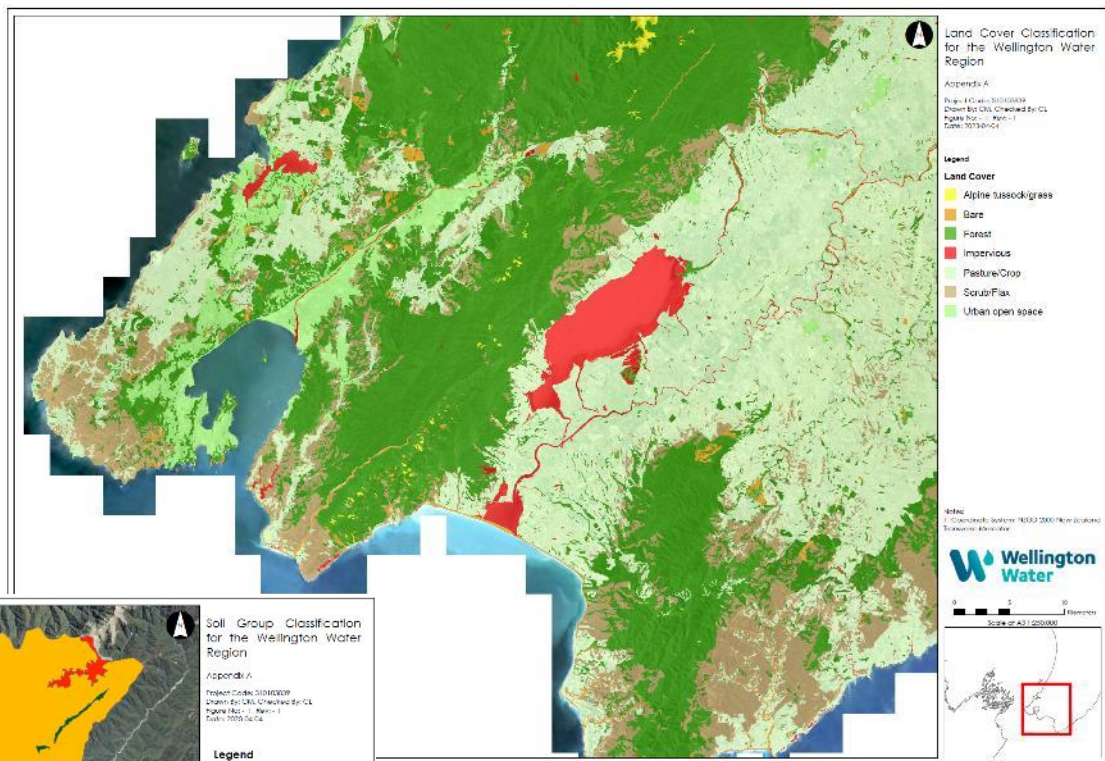


1. Curve number map > Soil group & land cover map
2. Sub-catchment delineation
3. Post-development reduction in soil group

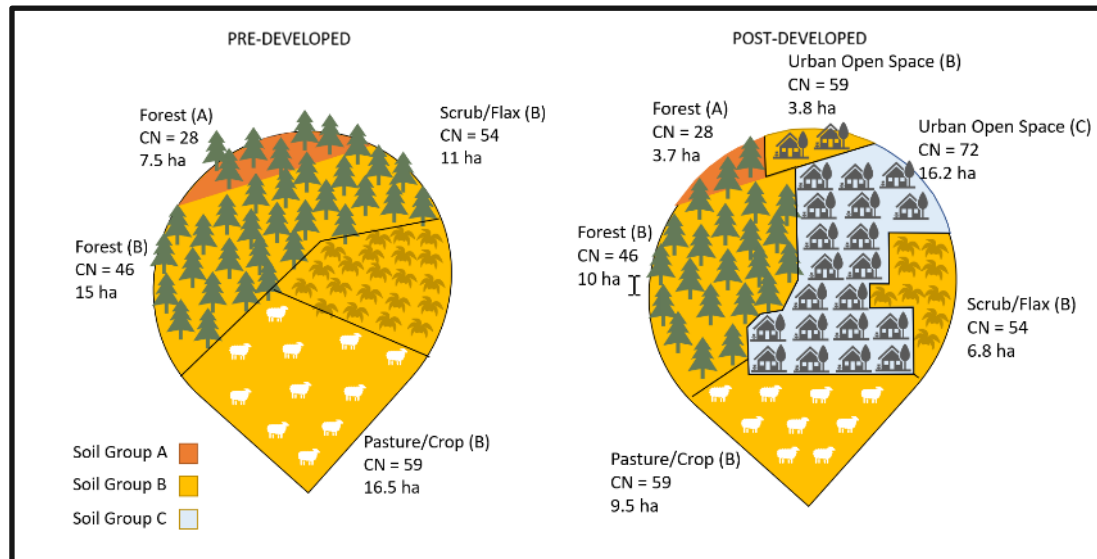
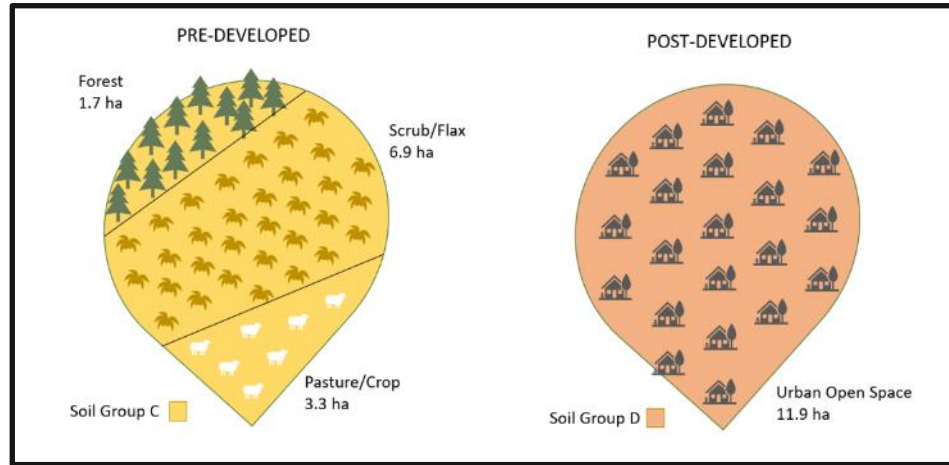
Gone from...



To...

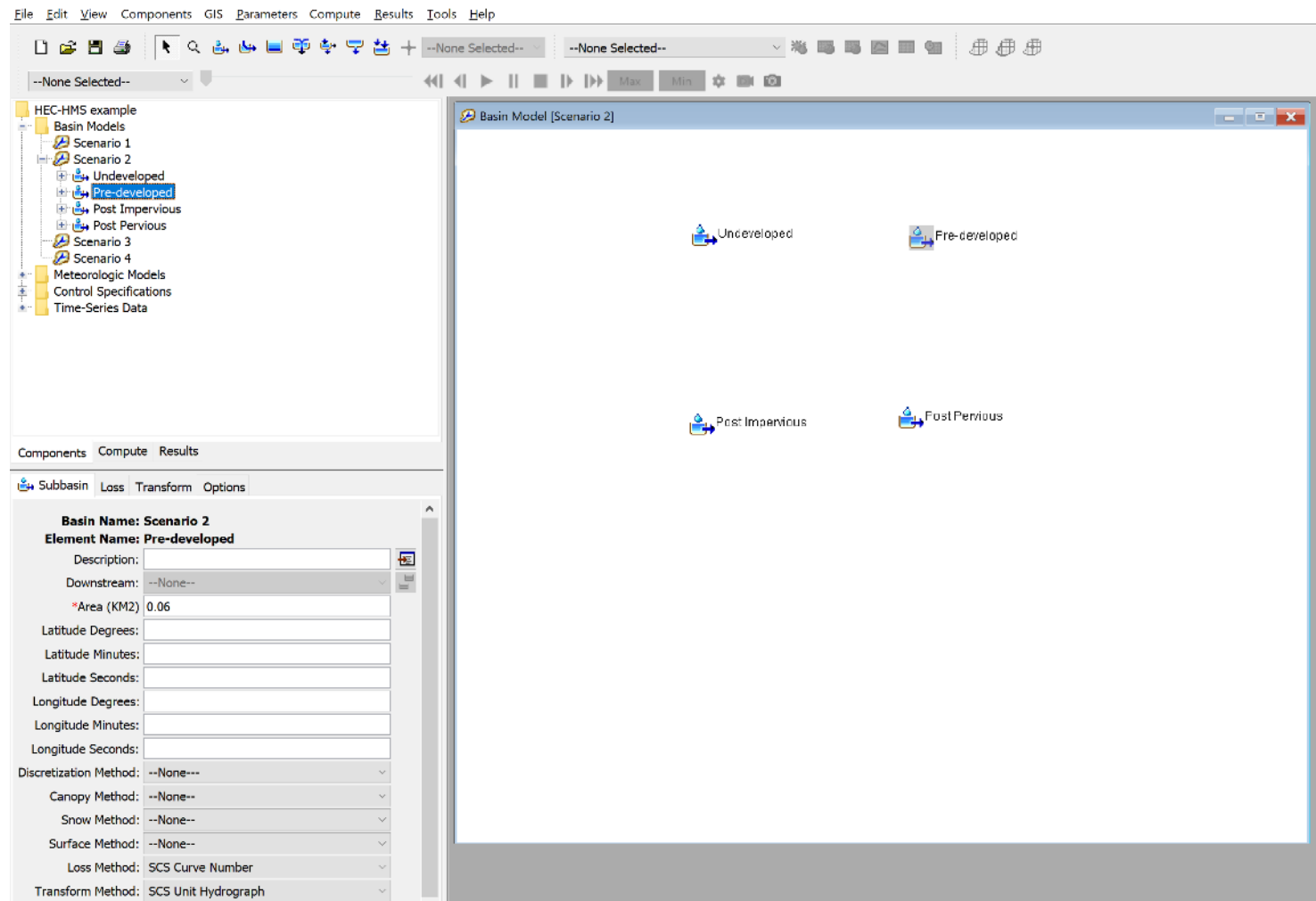


Sub-catchment delineation & reduction in post-development soil group



Runoff Hydrograph Calculator

The alternative



Runoff Hydrograph Calculator (RuHC)



WELLINGTON WATER LIMITED
Runoff Hydrograph Calculator
v1 Jan 2023



Input parameters to be derived from the methodology outlined in the Reference Guide for Design Storm Hydrology.

Site ID	Aotea
Event	1% AEP Climate Change
Area (km2)	0.0500
Initial Abstraction (mm)	21.6
Soil Group	C
Landcover	Forest
Curve Number default	63
Curve Number proposed	63
Time of Concentration (minutes)	10.0
12-hour rainfall depth (mm)	139.2
Peak flow (m3/s)	0.845
Volume (m3)	2590.7

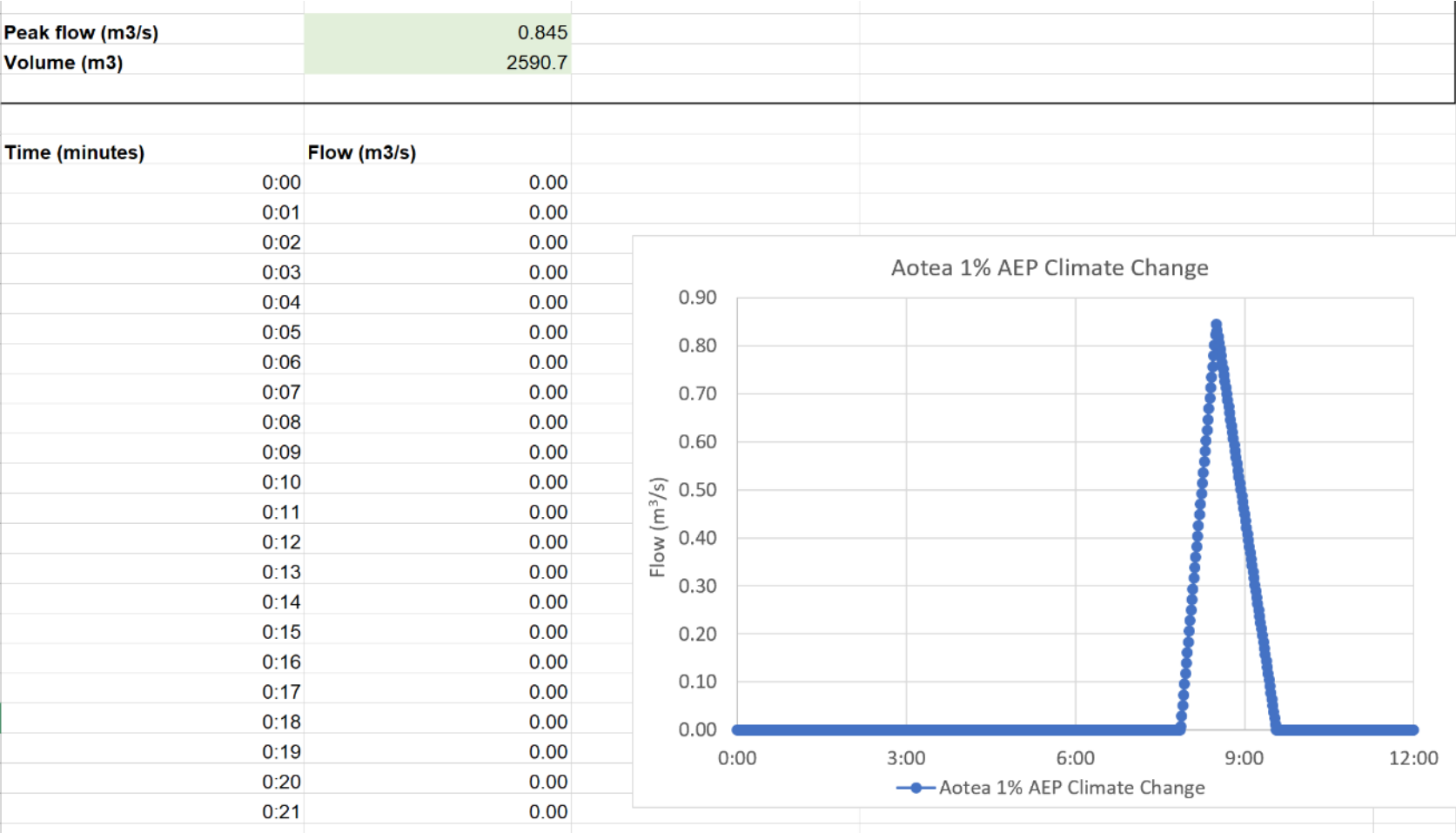
← ID & reference details

← Input parameters

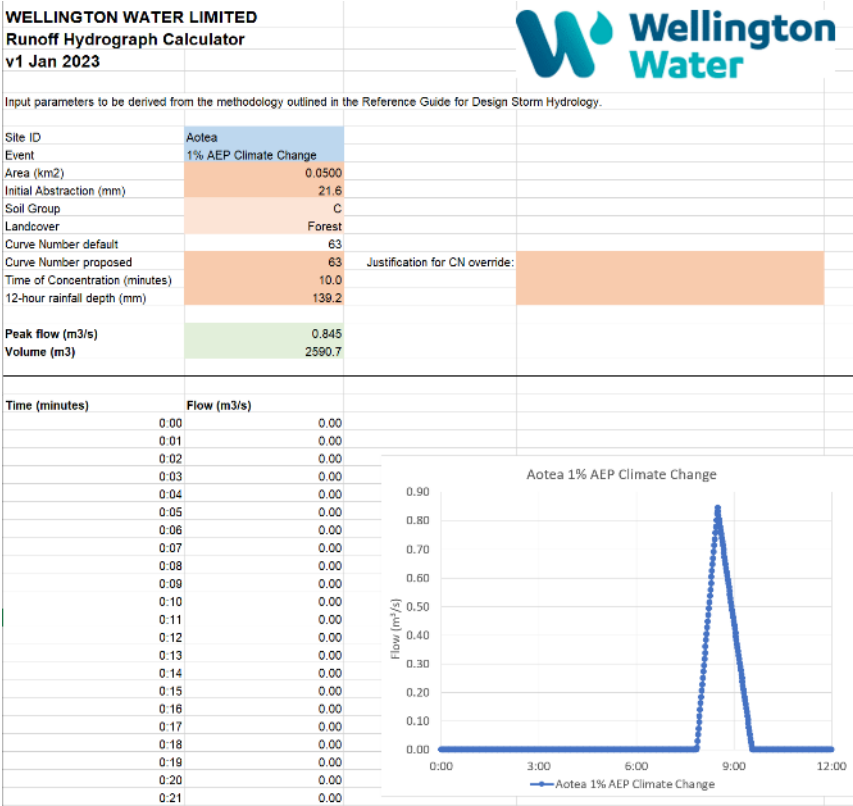
Justification for CN override:

← Results

Results



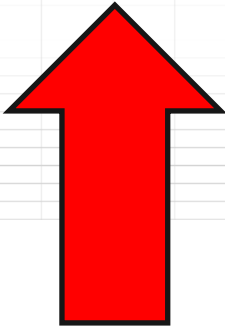
Disclaimer



This spreadsheet has been developed to estimate flood hydrographs for small ungauged urban catchments in the Wellington Region. It has been developed by Wellington Water Ltd (WWL) for use by developers and consultants for managing and designing stormwater infrastructure as an alternative to hydrological modelling in software such as Hec-HMS. The formulae in this spreadsheet are consistent with the *Reference Guide for Design Storm Hydrology*. For details on how to calculate the input parameters, please consult the *Reference Guide*.

While this methodology is consistent with the *Reference Guide*, assumptions have been made to convert site specific variables into a regional spreadsheet. For this reason the following limitations apply to use of this estimator.

1. Developed for small ungauged urban catchments in the Wellington area. The term 'urban' is to distinguish between large river catchments such as the Hutt River or Wainuiata Stream, and small tributaries/overland flowpaths/unnamed streams. It may be used to estimate runoff in areas where landuse is being developed from rural to urban.
2. Input parameters must be developed in accordance with the *Reference Guide*.
3. The maximum time of concentration has been capped at 120 minutes, beyond this definitely falls outside of the definition of a small ungauged urban catchment.
4. This is a simplified tool for simple calculations. For moderate to complex design, hydrological modelling using appropriate software is required. Hydrological modelling using appropriate software (with the methodology in accordance with the *Reference Guide*) can be used for all stormwater design and will supersede any outputs from this estimator.
5. It has been developed to size infrastructure with reference to larger flood events (i.e. 1% AEP flood). Where infrastructure is being sized for smaller flood events, or where there is significant storage within the catchment, hydrological modelling using appropriate software and taking into account site specific characteristics may result in smaller peak flow estimates.
6. The estimator provides a flood hydrograph for a single homogeneous sub-catchment. Where multiple sub-catchments are being evaluated, the resulting flood hydrograph across each scenario can be summed together to estimate the total peak flow and volume.
7. Hydrological modelling to estimate peak runoff rates are not highly accurate. The standard error associated with HIRDS v4 rainfall may be in the order of +/-20%. This error is further increased through hydrological modelling, and using this simplified calculator. If in doubt, apply conservatism.



Disclaimer

The technical stuff...

Excess runoff:

$$Q = \frac{(P - I_a)^2}{(P - I_a) + S}$$

where:

$$S = \left(\frac{1000}{CN} - 10 \right) 25.4$$

Peak flow:

$$q_p = q_u A Q F_p$$

Q = runoff volume (mm)

P = direct rainfall

I_a = initial abstraction

$P - I_a$ = maximum rainfall available for runoff

S = storage depth

CN = curve number

q_p = peak runoff

q_u = unit peak discharge

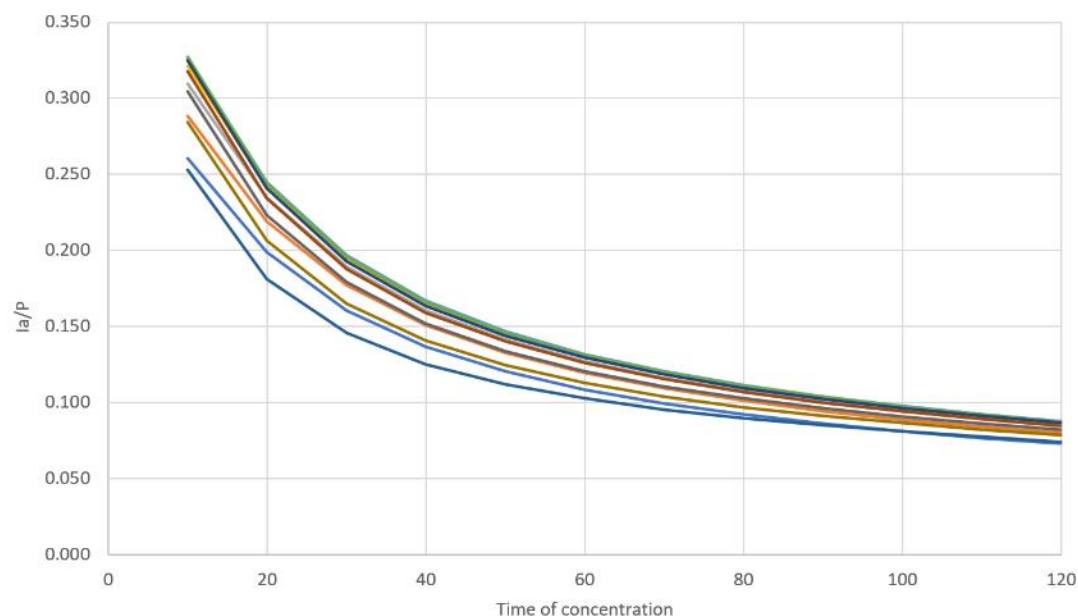
A = area

F_p = pond/swamp adjustment factor

The technical stuff...

- q_u decreases exponentially as time of concentration (T_c) increases and,
- for any given value of T_c , the value of q_u decreases with increasing values of I_a/P .

So the RuHC uses developed I_a/P vs q_u curves in the background to estimate Q_p .



Summary

The Wellington Water Ltd region now has a nifty calculator that takes the hard-yards out of estimating design flood flows*.

WWL to roll this out mid-2023.



*Note the assumptions & limitations to this approach.





Without change there is no innovation, **creativity**, or incentive for improvement. Those who initiate change will have a better opportunity to manage the change that is inevitable.

William Pollard

